

National Defense Industrial Association

2005 Tri-Service Infrastructure Systems Conference "Re-Energizing Engineering Excellence" August 2-5, 2005

# Iwakuni Runway Relocation Project

Vincent R. Donnally, P.E. NAVFA<del>C Atlantic CI Engineering 6506 Hampton Blvd.</del> Norfolk, Virginia 23508-1278 Vincent.Donnally@navy.mil 757-322-4204, fax-4416, cell 757-651-2857

# MCAS Iwakuni 2000/2010 – A Reality









NAVFAC Atlantic

8/22/2005

# Background



#### •US & Japan signed MOU Jul 1992

 -3 spheres of communication/oversight: Politicians, Regional & Senior Managers, & lastly, the project team

#### Primary reasons for project

Reduce noise in City of Iwakuni
Improve flight safety of Airfield

#### (Unstated) impact ....Commercial Use possibility

- Explosives Safety SEC CERT approved current plan in Mar 2000 revisions being reviewed
- GOJ project cost estimate \$2.4B
- Project in year 9 of 13
  - -Phase I reclamation complete
  - -Phase II reclamation in progress
  - -Port Area to complete in June 2005; Fuel Wharf in July 2005
  - -\$1.852B spent or authorized through JFY2005; \$2.082B projected after JFY2006
- Detailed Criteria Package conveying US military expectations

## **Project Background/Requirements (Continued)**



- •8000' Class B runway (with 1,000 foot overruns)
  - Hot-pit refueling
  - Simultaneous CALA and Red Label area operations
  - State-of-the-art aviation navigational aids
  - 1,000 foot overruns; US MCON must fund hardened overrun
  - Concrete pavement required due to heat signature of US military aircraft
     F/A-18, AV-8B,
- Port Facilities
  - 360 Meter General Purpose Wharf minimum 42' draft, 80T rail crane
  - 142 Meter North Breakwater Wharf minimum 60' draft, 100,000 lbs NEW
  - T-1 capable Fuel Wharf
  - New staging/container lay-down areas and warehouses.
- New Ordnance Storage Area eliminates explosives safety waivers

#### **Reclaimed Land: Sand Piles & Sand Drains Construction**





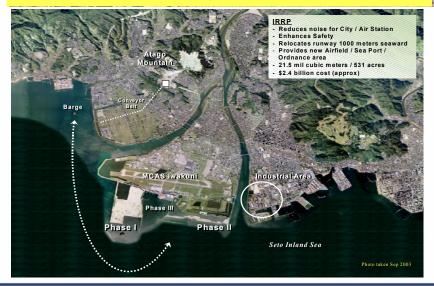
Crushed Rock Conveyed from Atago Mountain by Barge



SAND COMPACTION PILE AUGERS



Sand Piles: Liquefaction Prevention from Tight Consolidation Sand Drains: Uniform, Loose Consolidation



NAVFAC Atlantic

8/22/2005

#### **Construction Progress Photos**













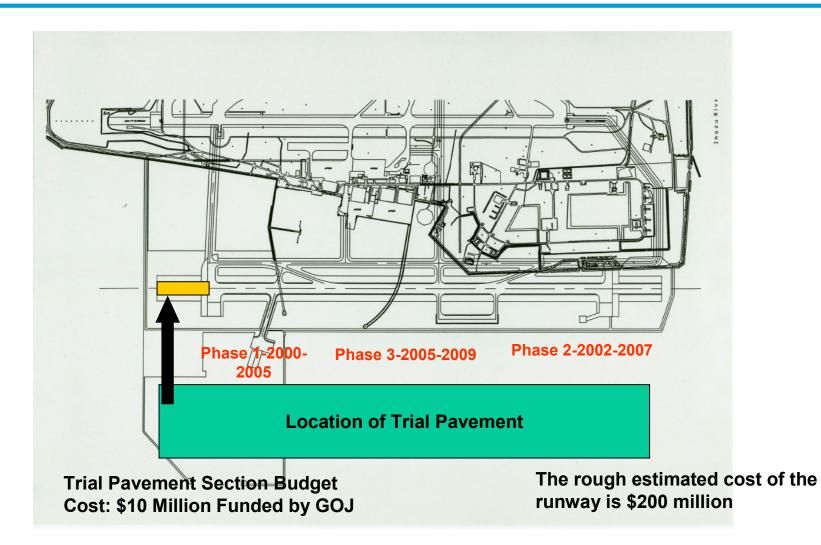
- •Flat Surface Criteria: This construction requires a smooth pavement for the runway on the reclamation area anticipating a consolidation settlement which will occur over many years.
- •<u>Settlement:</u>Uneven (differential) Settlement Amount: about 6cm at 50m intervals
  - -Sand Piles/Sand Drains
  - -3 phases of consolidation
- •<u>Unpredictable Soil Behavior</u>: The result of how the concrete pavement, compacted sub grade, and sand piles/sand drains will perform due to uneven residual settlement at reclamation area is not known at this time.
- •<u>Material Parameters</u>: Type of concrete pavement (CRC, reinforced concrete or NC, non-reinforced concrete) and base course and the final design thick nesses of concrete slab will be determined based on demonstration section results

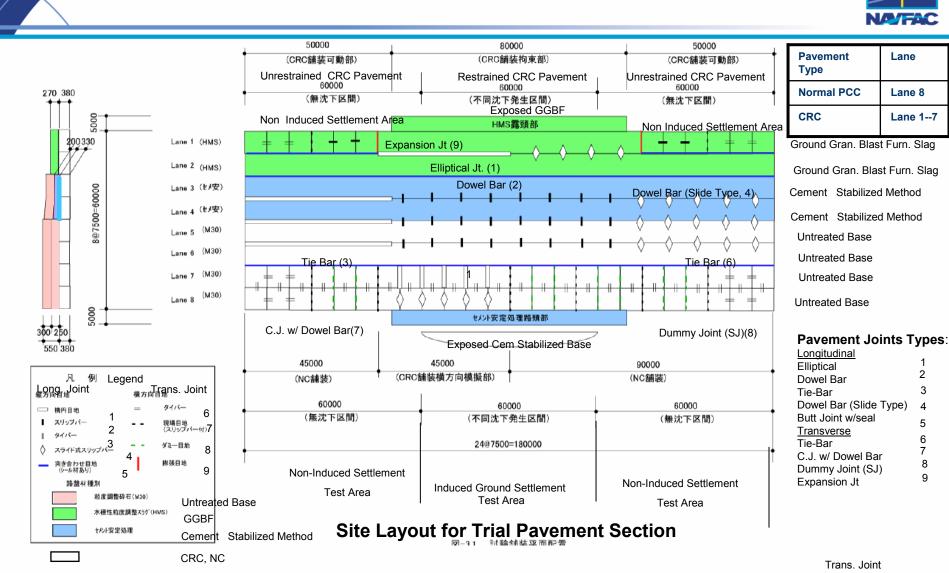
# Design Procedure Parameters:

- -GOJ developing their own design procedures; US to review using our tools. We need to verify what happens to the concrete slab during the test period
- Current concept of design cannot be verified until the testing of actual conditions occurs.
- Ground Settlement must be better understood by analyzed data gathered during test period for final pavement design

## **Test Section**







**Test Section** 

#### **TEST SECTION**











- •Purpose: The demo section will be used to determine the effects of artificially induced differential settlement and to understand concrete pavement behavior by applying repetitive loadings and other thermal, drying stresses.
- Pavement Loading
  - -747 Wheel cart
  - -Number of repetitions

Slab Instrumentation for load, thermal, & shrinkage stresses

Test Section Cost: \$10 million
GOJ/US Government to Use Demo results To Get Final Design Pavement Thickness Right!



## **PAVEMENT SENSITIVITY ANALYSIS**



PAVEMENT PROFILE CONDITION	LOADING CONDITION	NAVY PAVEMENT COMPUTED THICKNESS (Inches)	PCASE PROGRAM COMPUTED THICKNESS (Inches)
FLEX 650,SG 100	NAVY	15.0	15.7
FLEX 650,SG 200	NAVY	13.8	14.5
FLEX 650,SG 300	NAVY	13.2	13.2
FLEX 650,SG 500	NAVY	13.0	11.2
FLEX 650,SG 100	AIR FORCE	27.2	20.3
FLEX 650,SG 200	AIR FORCE	25.6	16.8
FLEX 650,SG 300	AIR FORCE	24.7	13.8
FLEX 650,SG 500	AIR FORCE	24.6	11.8
FLEX 726,SG 100	NAVY	14.1	14.8
FLEX 726,SG 200	NAVY	12.8	12.5
FLEX 726,SG 300	NAVY	12.3	10.6
FLEX 726,SG 500	NAVY	12.2	10.7
FLEX 726,SG 100	AIR FORCE	25.6	19.0
FLEX 726,SG 200	AIR FORCE	24.1	15.6
FLEX 726,SG 300	AIR FORCE	23.3	12.9
FLEX 726,SG 500	AIR FORCE	23.1	11.1

NAVFAC Atlantic

8/22/2005

# **Pavement Structure**



ltem	Sub Item	Test Condition		Remarks
Classification of Pavement		NC,CRCNC Pavement, CRC Pavement		
Concrete	Design Basis Flexural Strength	5N/mm²		
	Elastic Coefficient	2.75×10⁵N/mm²		
	Poisson's Ratio	0.15		
NC Pavement	Slab Thickness	38		C-141 Objective Aircraft
CRC Pavement	Slab Thickness	33cm w/ Cement Treated Base	38cm	C-141 Objective Aircraft
	Amount of Rebar for Longitudinal Direction	D19ctc13cm	D19ctc11.5cm (D22ctc15.5cm)	
	Amount of Rebar for Lateral Direction	D13ctc37cm	D13ctc32cm	
	Depth of Rebar for Longitudinal Direction	<b>11cm</b> 11cm from Top Surface	<b>12.5cm</b> 12.5cm from Top Surface	

# **Pavement Structure**



Base Course Materials	Granular Material Untreated Base	Cement Stabilization	нмs Hydraulic Mechanical Stabilized Slag (GGBF)	
Select Materials	Granular Material I CBR Modified CBR <b>80%</b> Granular Material II CBR Modified CBR <b>&lt;80%</b>	Cement Stabilization E=5,000N/mm <sup>2</sup>	HMS E=750N/mm <sup>2</sup>	
Coefficient of Bearing Factor for Design Sub	K75= 24 (MN/m <sup>3</sup> )			
grade Coefficient of Bearing Factor for	K75= 70	150	70	
Base Course Coefficient of Bearing Factor for Design Base	K75= 56	120	56	

# **Questions?**



