PROJECT UPDATE



US Army Corps of Engineers ® Jacksonville District

- Alberto Gonzalez, P.E. Project Manager
- Jim Mangold, P.E. Project Engineer
- Dave Dollar, P.E. Structural Designer
- Geotechnical, Geology, Materials, Hydraulic, Civil, Mechanical, Electrical, ITR Team



- Jim Hinds CENWP RCC Mix Design
- Tony Bombich and Billy Neeley CEERD – Materials Testing
- Ahmed Nisar, Paul Jacob MMI Engineering – Thermal Stress/Strain Analysis (NISA)







August 3, 2005

- I. Project Overview
- II. ITR Process
- III. Current Schedule
- IV. MCE Update
- V. Dam Design





I. Project Overview CHANNEL IMPROVEMENTS CONCRETE U-CHANNEL GABION LINED UNLINED DROP STRUCTURES CONTROL STRUCTURES DEBRIS BASINS CERRILLOS DAM

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PORTUGUES & BUCANA

RIVERS PROJECT





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Concrete Thin Arch Dam was advertised in September 2000 and the bid was outside the awardable range

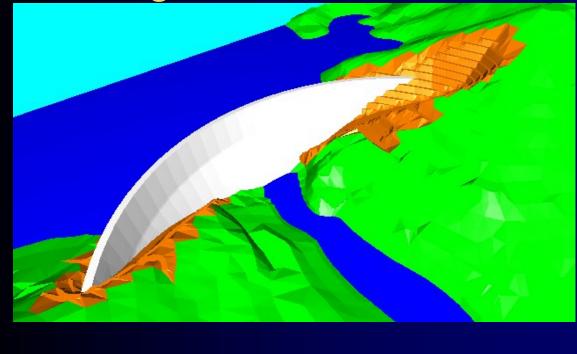
Design changed to RCC

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Pertinent Data:

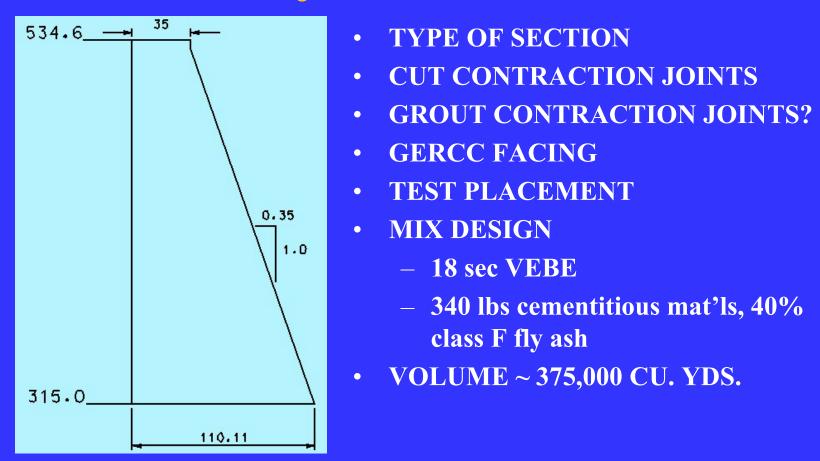
- HEIGHT: 219.6 FT
- CREST LENGTH: 1300 FT
- SPILLWAY CREST WIDTH: 150 FT*
- FLOOD CONTROL STORAGE: 9484 AF
- MAX POOL AREA: 215 ACRES

Portugues Dam - Thick Arch









PORTUGUES DAM II. ITR Process

- THIN ARCH
- RCC

- FORMALIZED PROCESS

 CONSISTENT WITH INDUSTRY PRACTICE

PORTUGUES DAM II. ITR PROCESS

- Multidiscipline ITR team.
 - Concrete dam design, RCC mix design, seismology of the Caribbean, engineering geology, geotechnical engineering, hydraulics, electrical and mechanical engineering.

PORTUGUES DAM II. ITR PROCESS

Multidiscipline ITR team: Concrete dam design RCC mix design Seismology of the Caribbean Engineering geology Geotechnical engineering Hydraulics Electrical engineering Mechanical engineering Individuals: Glenn Tarbox Gary Mass Dr. William McCann Alan O'Neil Dr. Gregg Korbin, Dr. Don Banks MWH staff MWH staff

PORTUGUES DAM III. Current Schedule*

- COMPLETE P&S MAY 2006
- ADVERTISE MAY 2006

AWARD – AUG 2006 *THIS SCHEDULE IS DEPENDENT ON AVAILABILITY OF PROJECT FUNDING

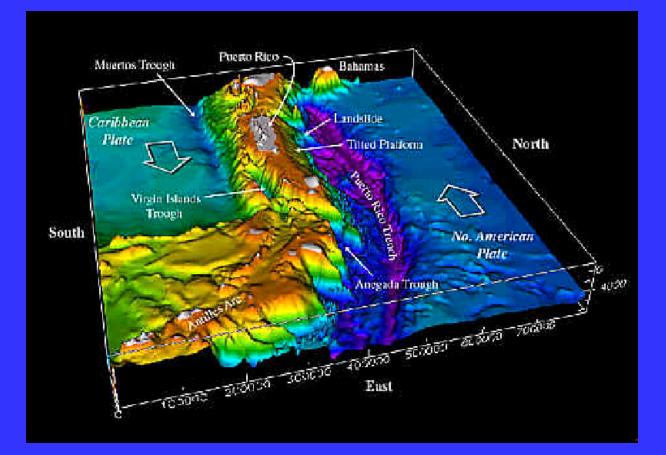
PORTUGUES DAM IV. MCE Update

MCE – Controlling Events:

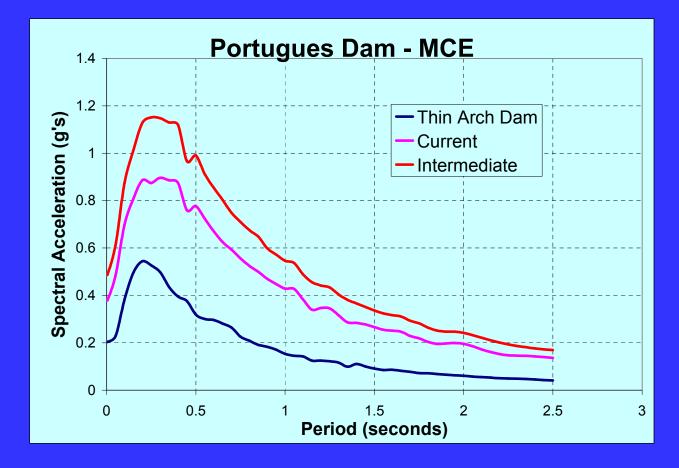
- Thin Arch Dam
 - M6.5 @ 18km Salinas Fault 1988
- RCC Thick Arch Dam
 - M8.25 @19.6km Muertos Trough 2004

"Deterministic and Probabilistic Seismic Hazard Analysis for Portugues Dam, Puerto Rico," 6 April 2004, prepared by URS Corporation; reviewed by ITR Team (particularly Dr. William McCann), Dr. Greg Fenves, ERDC (Dr. Donald Yule), USGS (Dr. Charles Mueller)

REGEONAL GEOLOGY



PORTUGUES DAM IV. MCE Update



PORTUGUES DAM IV. MCE Update

Significance to dam design:
➢ Peak ground acceleration: 0.38g's.
➢ Plateau on the response spectrum throughout the range concrete dam frequencies of vibration.

Sequencing of Design Activities:

Construction for the thin arch dam had begun(excavation & grout curtain); therefore, there was a need to minimize the time required to redesign the dam. Activities that would normally run sequentially were performed in parallel.

Parallel Activities:

➢ Site Seismicity

> Determination of Foundation Properties

Foundation and Slope Stability

Concrete Mix Design and Property Testing

≻Dam Design

≻Thermal Analysis

PORTUGUES DAM V. Dam Design DISADVANTAGES OF PARALLEL ACTIVITIES ACTIVITY INPUT REQUIREMENTS

1. Dam Design

- 1. Foundation Properties, Seismic Input, Concrete Properties.
- 2. Foundation Stability
- 3. Thermal Analysis
- 4. Mix Design

- 2. Dam Shape and Loads, Seismic Input
- **3. Dam Shape, Construction** Sequencing, Concrete Properties
- 4. Target Parameters

Design Approach:

Based on expected magnitude of seismic loading; design a workable mix with reasonable bond strength (tensile strength) and design the dam to maximize cantilever compression on the upstream face under usual loadings and arch compression during the seismic loading.

Design Progression:

- Corps experience with RCC has typically been associated with gravity dams.
- The district considered an RCC gravity structure in the 1980's but ruled it out, not based on cost, but on the "newness" of the technology.

Design Progression:

Gravity dam alignments and sections were evaluated.

Detailed cost estimates, which included the quantities of RCC and excavation for the gravity dam designs, indicated a cost savings compared to the thin arch dam.

Design Progression:

- Now that a more economical construction method was adopted could further savings be realized by minimizing the volume by designing a thick arch structure?
- Preliminary layouts indicated that a thick arch dam could be designed with less than 3/4 the volume of the gravity dam.

Design Progression:

To maintain simplicity during construction a section was adopted with a vertical u/s face and a d/s face with a single slope.

> Sensitivity analyses were performed to evaluate:

- Relative stiffness of the arches and cantilevers
- Effect of varying the horizontal curvature
- Effect of stiffening the upper arches
- Magnitude of temperature and reservoir load compared to gravity load

Design Progression:

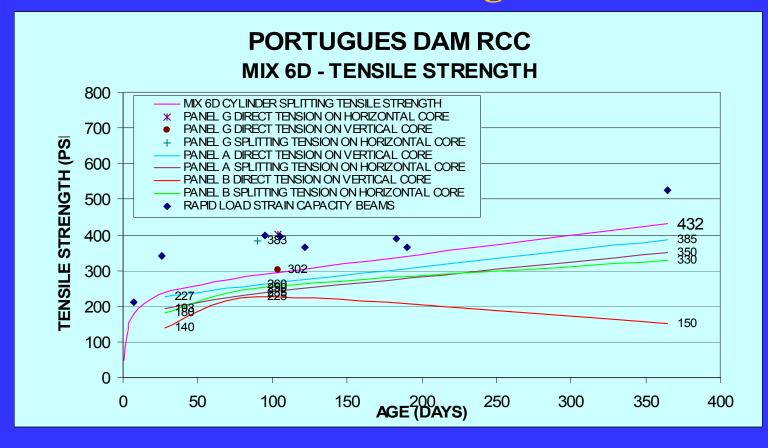
Based on the water supply dam, a full reservoir and the foundation properties from the thin arch analysis; the horizontal curvature and alignment were set prior to having the final seismic loading. The left abutment was shifted upstream to avoid highly weathered rock exposed during the thin arch excavation.

Design Progression:

- The section was refined to increase u/s cantilever compression; mainly from gravity load, which was applied to cantilevers only.
- The final layout was selected and a dynamic analysis performed.
- > The dynamic response was acceptable.

Design Progression:

- The foundation properties were determined for the final layout. (In progress)
- All load cases analyzed for the final properties and loadings. (In progress)



 LAYOUT:
 VOLUM

 G - Raxis = 825 ft, S=0.50, Crest Thickness = 25 ft
 257710 G

 H - Raxis = 825 ft, S=0.40, Crest Thickness = 30 ft
 356284 G

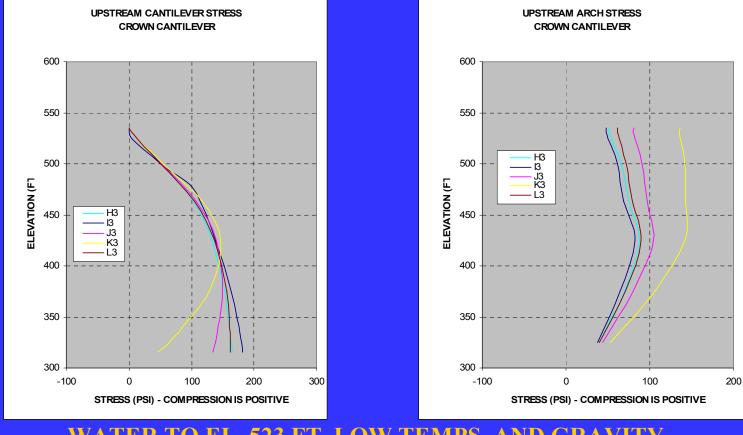
 I - Raxis = 825 ft, S=0.40, Crest Thickness = 35 ft
 379937 G

 J - Raxis = 825 ft, S=0.30, Crest Thickness = 35 ft
 343610 G

 K - Raxis = 825 ft, S=0.20, Crest Thickness = 35 ft
 301013 G

 L - Raxis = 825 ft, S=0.35, Crest Thickness = 35 ft
 367141 G

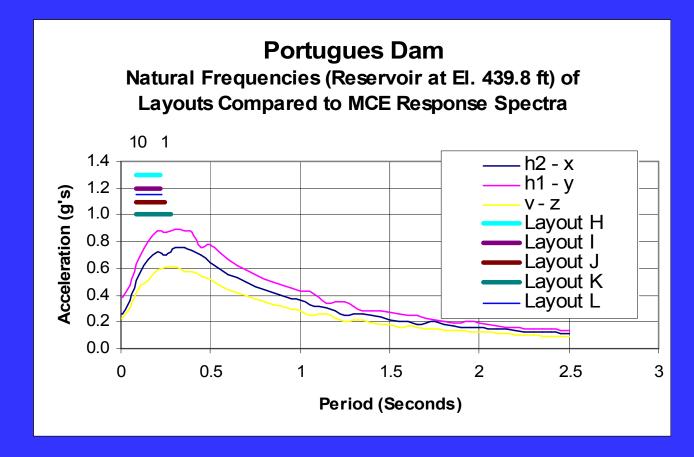
VOLUMES: 257710 CU.YDS. 356284 CU.YDS. 379937 CU. YDS. 343610 CU.YDS. 301013 CU.YDS. 367141 CU. YDS.



WATER TO EL. 523 FT, LOW TEMPS, AND GRAVITY

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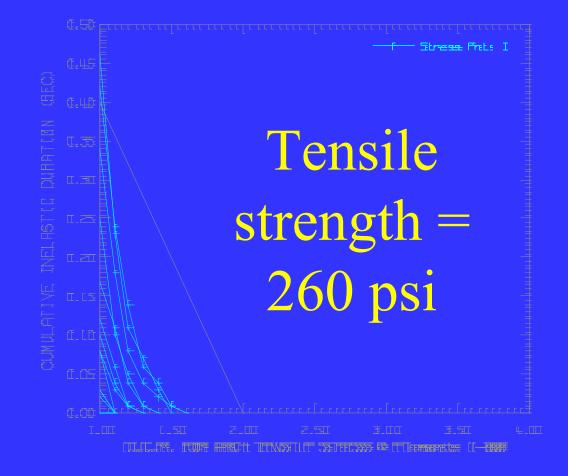
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• MAXIMUM TENSILE STRESSES

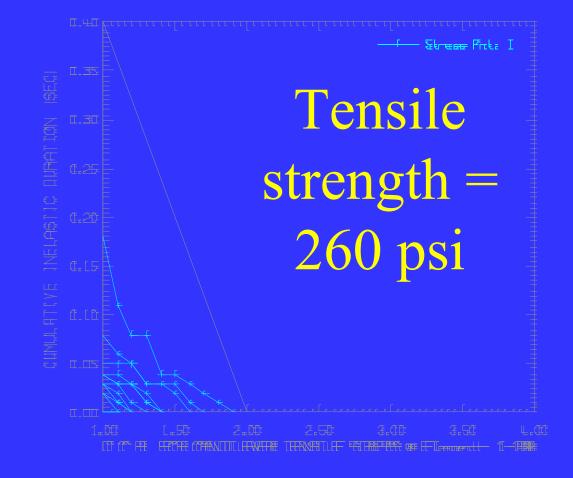
- #1- 61,Dir: u/s,Str:arch , Max: 399.474 @ 14.010Sec
- #1- 53, Dir: u/s, Str:cantl, Max: 476.163 @ 20.240Sec
- #1-296,Dir: d/s,Str:arch , Max: 249.882 @ 14.010Sec
- #1-271, Dir: d/s, Str:cantl, Max: 384.474 @ 20.370Sec

PORTUGUES DAM V. Dam Design–Demand/Capacity Curves



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PORTUGUES DAM V. Dam Design–Demand/Capacity Curves



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Factors affecting dam design:

- Earthquake loading
- Much of the dam design work and mix design preceded the determination of the earthquake loading

Tensile strength of RCC structures
Post thin arch excavation site conditions
Use of existing thin arch grout curtain

Factors affecting dam design (continued):

- Horizontal curvature compatible with either a flood control or water supply dam
- > Need axis before MCE was determined
- Left abutment weathered rock
- Delays and costs associated with exploration upstream of the thin arch left abutment
- Mix design program preceded determination of MCE.

THANK YOU

• RCC CONSTRUCTION PHOTOGRAPHS

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RCC Placement – Upper Stillwater



RCC Placement - Olivenhain



RCC Placement - Olivenhain



RCC Placement - Saluda



RCC Placement - Saluda



Cutting Contraction Jt. - Olivenhain



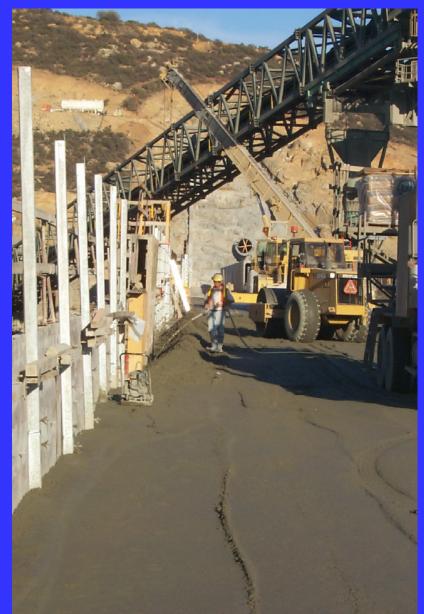
Cutting Contraction Jt. - Olivenhain



Cutting Contraction Jt. - Saluda















Batch Plant - Saluda



Aggregate Cooling - Saluda



Quarry - Saluda



Pre-cast Facing Panels - Saluda



Pre-cast Facing Panels - Saluda



Contraction Joint Details - Saluda



Contraction Joint Details - Saluda



THANK YOU

- Dave Dollar, P.E. Structural Designer
- Jim Mangold, P.E. Project Engineer
- Alberto Gonzalez, P.E. Project Manager (904) 232-2459



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