

> 2005 Tri-Service Infrastructure Conference St. Louis, Mo. August 4, 2005

Design of Concrete Lined Tunnels in Rock

CUP McCook Reservoir – Distribution Tunnels Contract

David Force, SsE



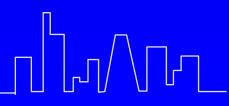
Outline of Presentation

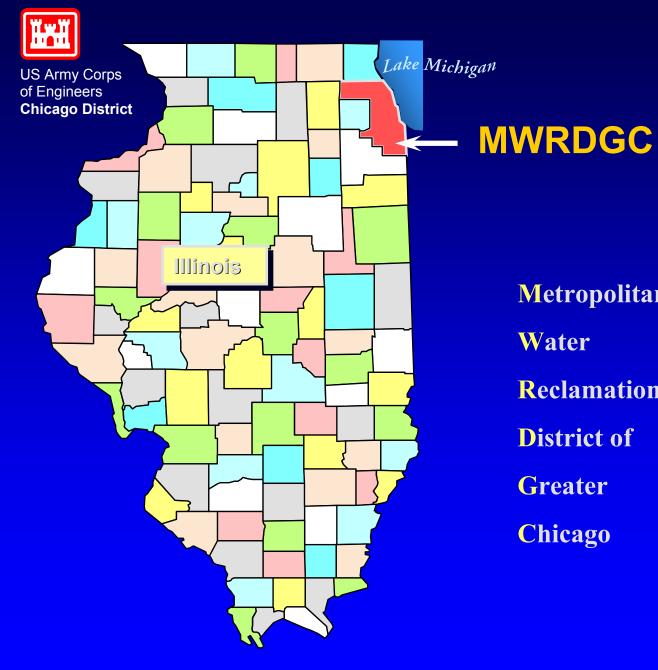
- General Project overview McCook Reservoir Project
- Overview of Distribution Tunnels Contract
- Design of Circular Tunnel Lining on Distribution Tunnels Contract
- Design of Concrete Bifurcations on Distribution Tunnels Contract
- Overview of Steel Liner Design on Distribution Tunnels Contract





McCook Reservoir Project





Metropolitan

Reclamation





Overall Goal – Control Flooding and Keep CSO Out of Lakes and Rivers !



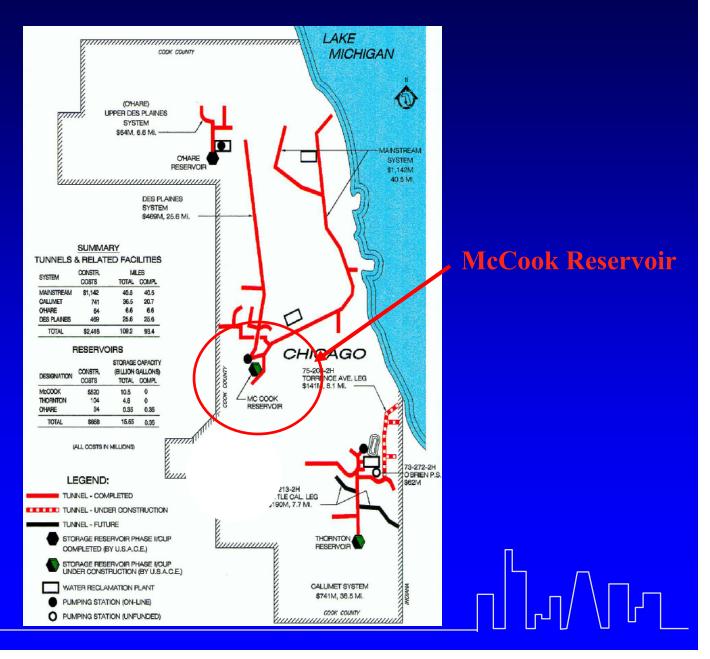
McCook Reservoir

- Estimated cost \$520 million
- Provides flood control between Des Plaines River and Chicago Sanitary and Ship Canal
- Excavation of reservoir will be by Drill and Blast (Quarrying)
- Captures CSO's from Chicago and 37 suburbs
- **Provides > 10 billion gallons of storage**
- Scheduled Project Completion FY 2012

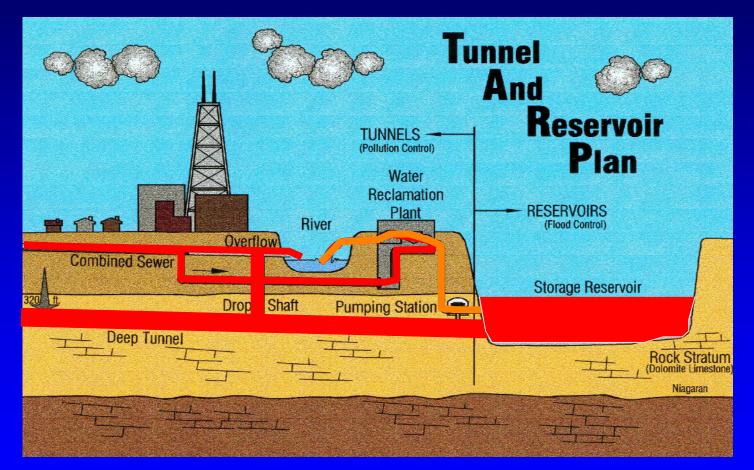
TARP / CUP SYSTEM

US Army Corps of Engineers Chicago District

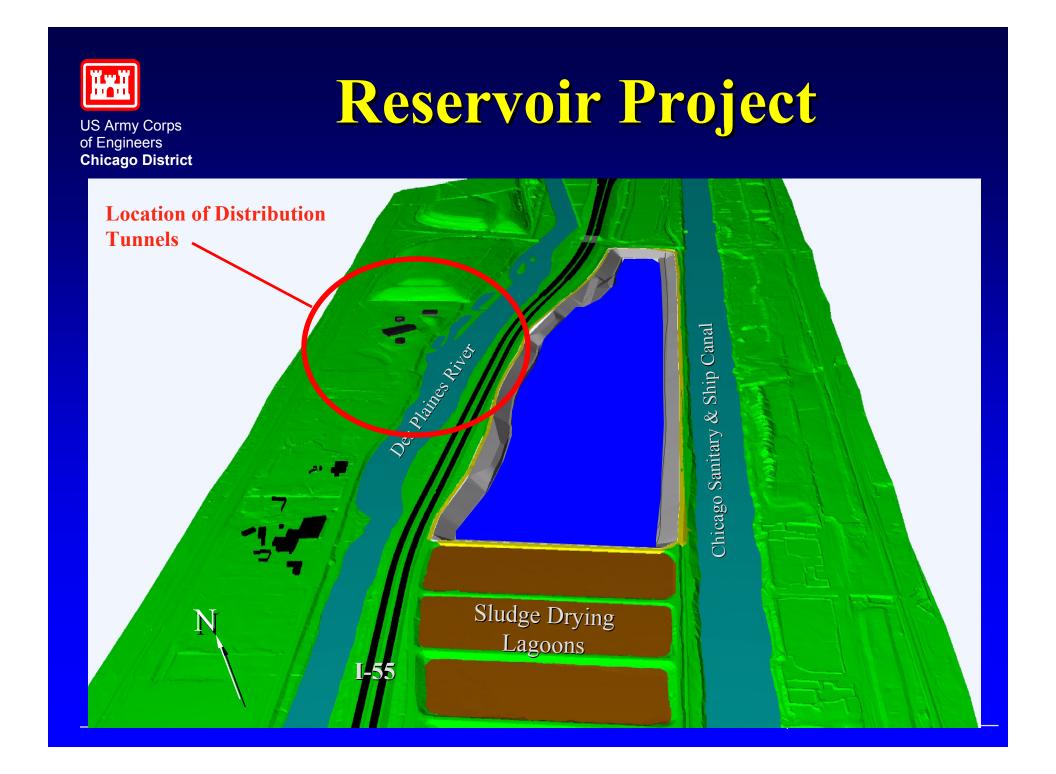
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Distribution Tunnels Contract





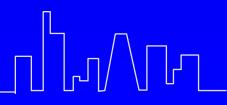
Distribution Tunnels Contract

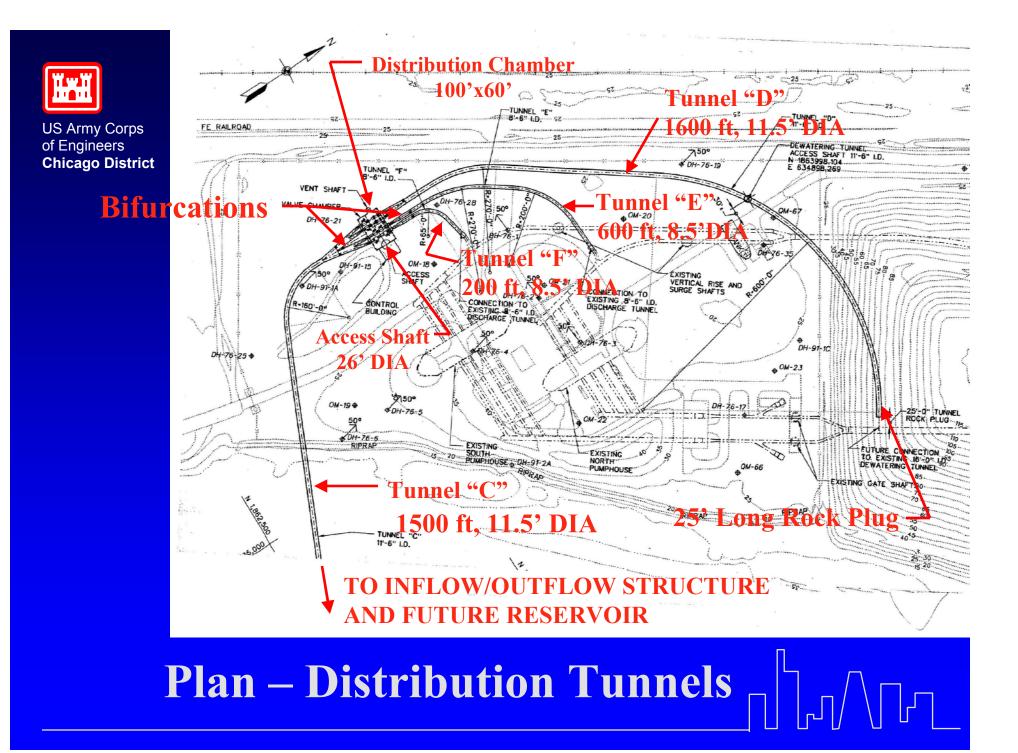
- LS: Metropolitan Water Reclamation District of Chicago (MWRD)
- Designer: Montgomery Watson Harza
- Construction Contractor: Kenny Construction
- Gate Designer: INCA (sub to Kenny)
- Steel Liner Fabricator: *CBI (sub to Kenny)*



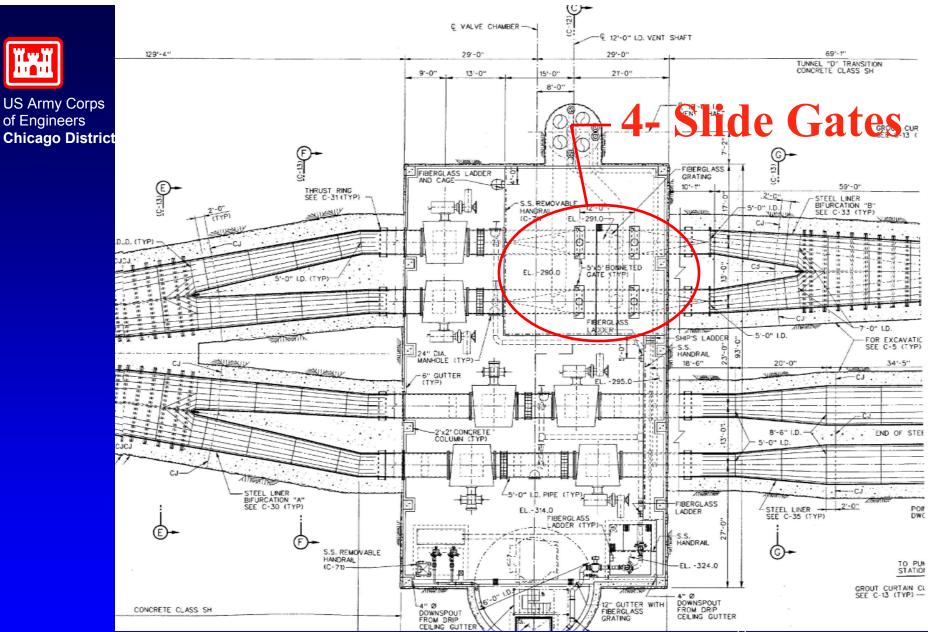
Purpose of Distribution Tunnels

 Convey and Distribute CSO's between the new Reservoir and the existing TARP Pump Stations and Tunnels

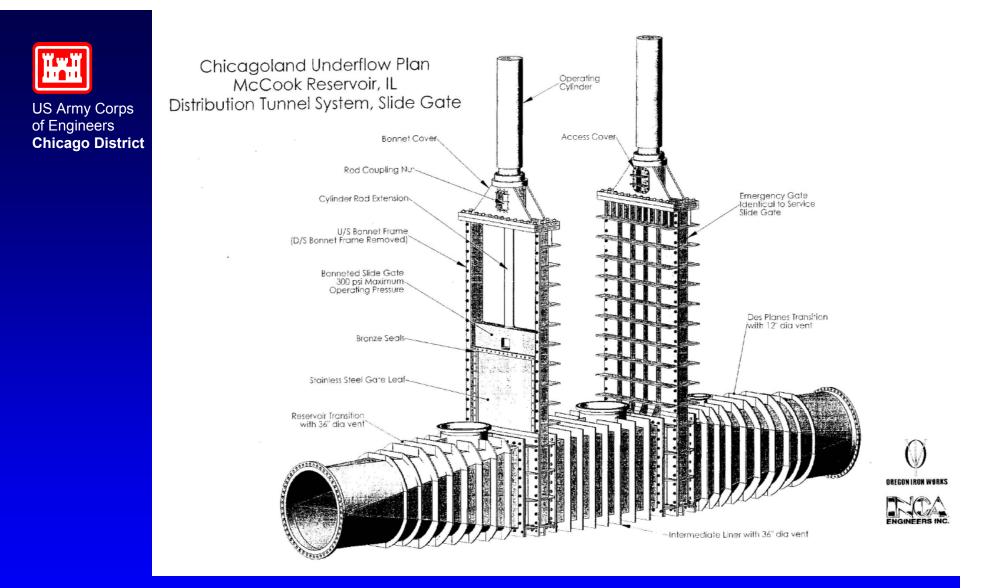








Distribution Chamber



Bonneted Slide Gates – 5'x 5'



CONTRACT COST/SCHEDULE

US Army Corps of Engineers **Chicago District**

Total contract Completed Anticipated Completion Date:

\$60 million 85% Jan 2006





Design of Circular Tunnel Lining







EM 1110-2-2901 30 May 1997

ENGINEERING AND DESIGN

Tunnels and Shafts in Rock



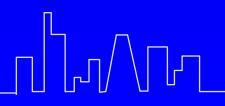


ENGINEER MANUAL

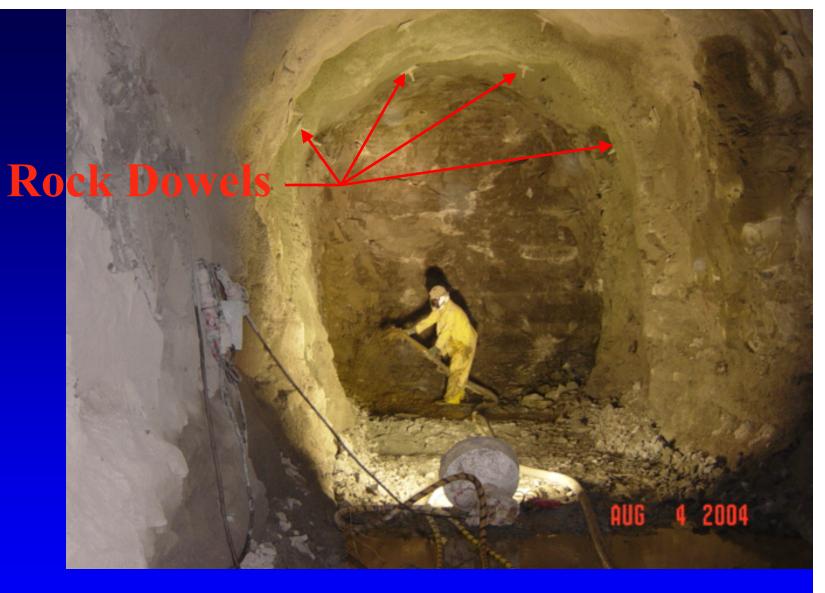


Tunnels General

- 3100 Lineal Feet of 11.5' DIA. Tunnel
 800 Lineal Feet of 8.5' DIA. Tunnel
- Approximately 310' below grade
- Excavation by Drill and Blast Creating a horseshoe shaped excavation







Tunnel Excavation – Drill and Blast

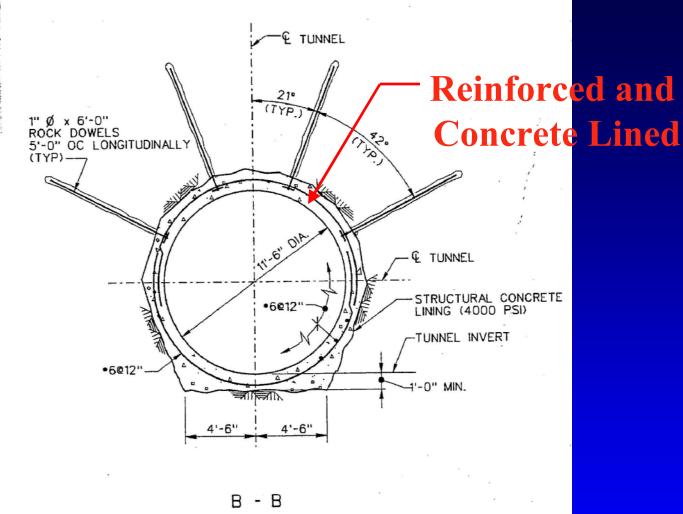


Tunnels General (con't)

- Final Tunnel cross sections are Circular except at bifurcations.
- At bifurcations cross sections are oblong or vary between circular and oblong







TUNNEL CROSS SECTION

SCALE: 14"= 1'-0"

Typical Tunnel Cross Section



Why Reinforced?

Most of the Chicago TARP tunnels are not reinforced because;

- Exfiltration is not a concern since external pressures from ground water exceed internal pressures





Why Reinforced? (con't)

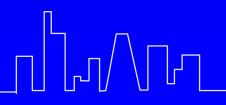
On Distribution tunnels reinforcement is provided because;

- The proximity of the reservoir draws groundwater down allowing exfiltration





- Velocities > 100 fps can occur around gates and valves in tunnels – those areas are steel lined and backed with 6000 psi concrete
- Tunnel C and D are low velocity gravity 4000 psi concrete





Design Loads Circular Tunnel Liners

Internal Pressures

Max Hydraulic Dynamic Pressure of 160 psi

• External Pressure

Hydrostatic Load from Ground Water

head = 310 ft or 132 to 134 psi





Key Design Assumptions

- All rock loads are assumed to be fully supported by permanent rock dowels. No rock loads to the liner.
- Relaxation of the rock and stress redistribution is assumed to occur prior to installation of the lining





Crack Width Limitation (Internal Pressure Design)

Crack Width Limited to .008" for water tightness

• Tensile stresses in the reinforcing are limited to limit the crack width.



Materials

• Concrete strength:

4000 psi in tunnels6000 psi around steel liners10,000 psi at concrete bifurcation

• Reinforcing:

ASTM A615, GR 60





Analyses Procedure

Tunnel Lining is analyzed for Internal External pressure



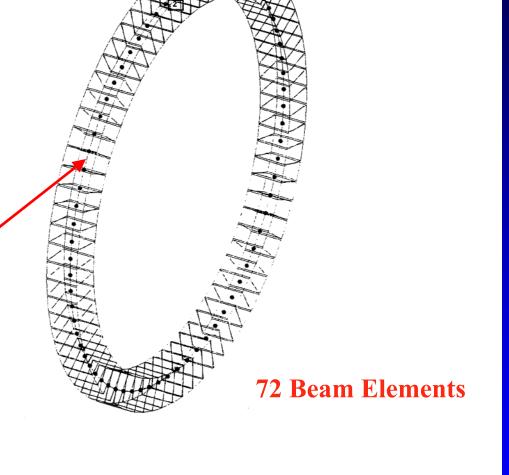


External Pressure Design Procedure

1. Determine and apply external pressures: 132 psi for 11.5' diameter tunnels 2. Determine Load Case(s): 1.1 D + 1.4 H (EM 2901, Table 9-1) **3. Model tunnel Lining using STAAD** 4. Design Concrete for Hoop Compression



Tunnel Lining modeled with beam elements ——



11.5 ft I.D. Tunnel

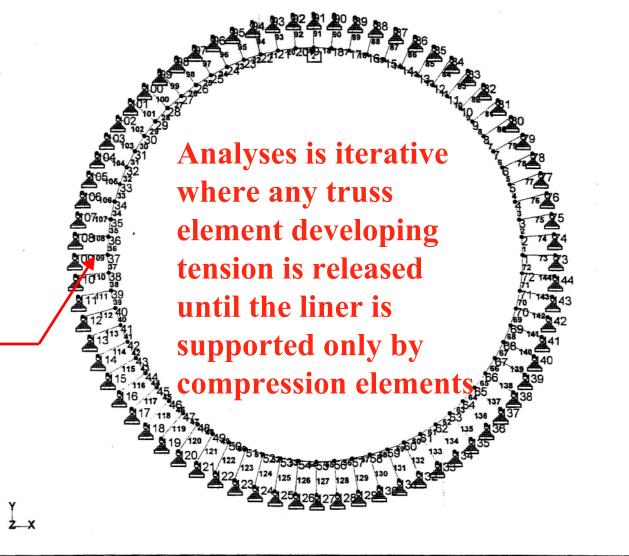
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STAAD FE Model



Rock Modeled With truss elements —

Radial spring Stiffness assigned Per Equation 9-18, EM 2901.

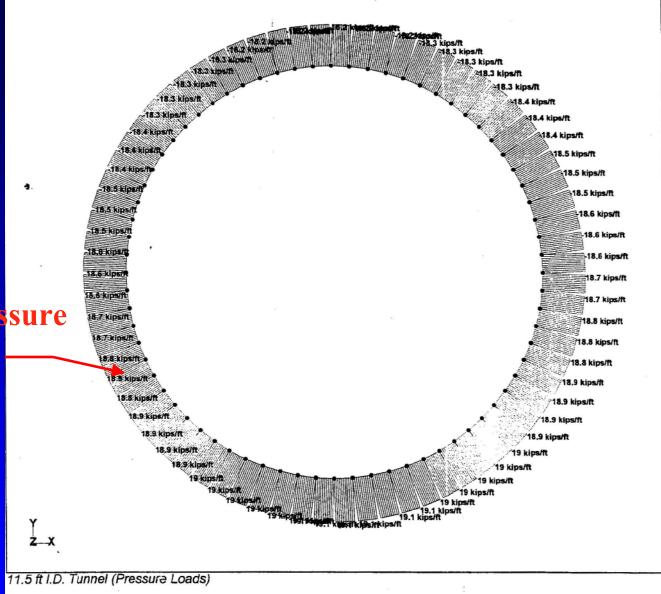


11.5 ft I.D. Tunnel (Beam and Node Numbers)

STAAD Model



External Pressure Load 132 psi



STAAD Model



Results – External Pressure Design

Primary Load is hoop compression Pu = 164 K/FT for 11.5' Tunnels

• Moments and Shears are negligible



Internal Pressure Design Procedure

- 1. Determine and apply internal pressures: 160 psi11.5' diameter tunnels
- 2. Determine Load Case(s): 1.1 D + 1.4 H (EM 2901)
- **3. Model the tunnel using Program "TUNNEL" developed by MWH.**
- 4. Design Reinf. to Limit crack width to .008"



Model Features (Internal Pressure Design)

- 1. Surrounding Rock Mass was modeled as a thick walled cylinder
- 2. Deformation properties of the concrete lining and sound and fissured rock were modeled.
- **3. Strain compatibility was performed to determine** % of load carried by the rock and the lining.



Rock Properties (Internal Pressure Design)

- A 40" ring of fissured rock was modeled due to drill and blast excavations.
- Then, sound rock was modeled beyond the fissured zone

Fissured Rock (grouted)Erock = 480,000 psi

Sound RockErock = 1,300,000 psi



Results

(Internal Pressure Design)

• Primary Load was tensile stress in the Concrete.

Maximum Tensile Stress = 600 psi

- Reinforcement was sized to limit crack width to .008 inches
- Resulted in #6 @12 inches



Rock Dowels

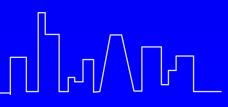
Setting Forms



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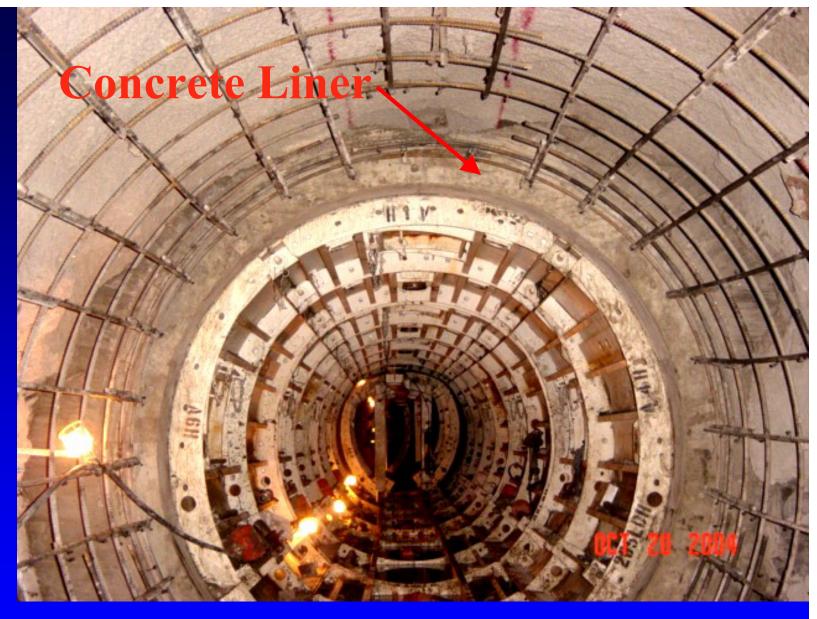






Window in Forms for Concrete Placement





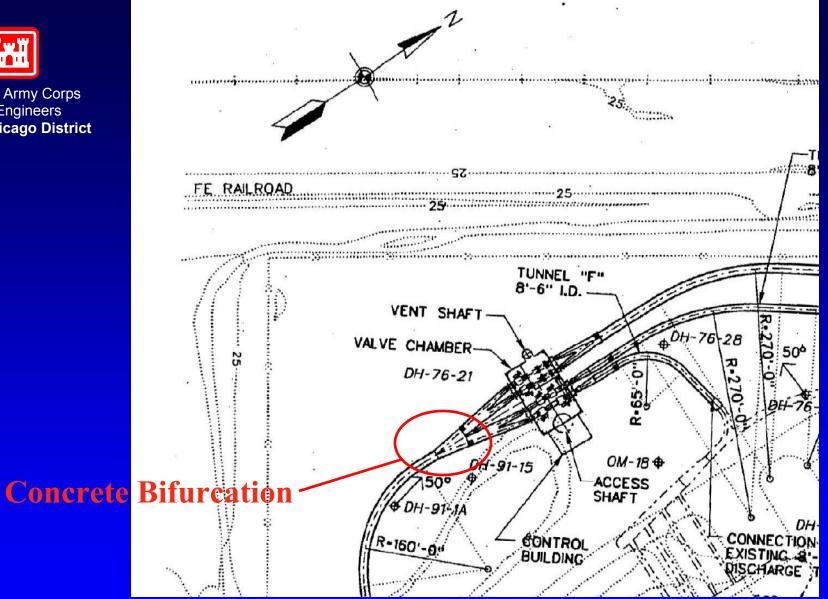
Tunnel Lining Formwork



Design of Concrete Bifurcations

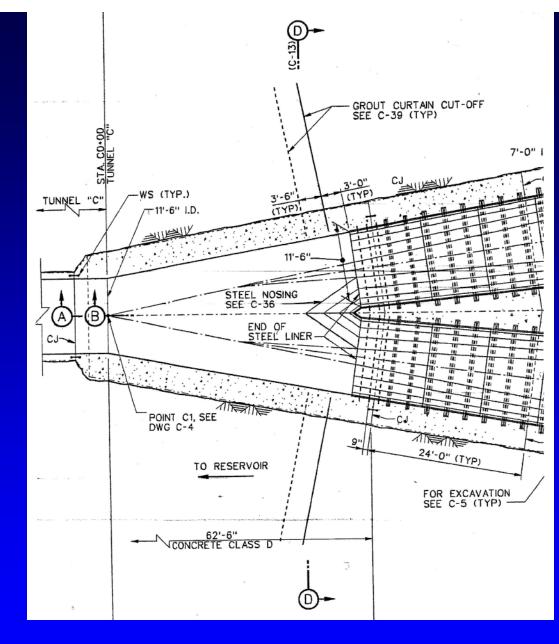






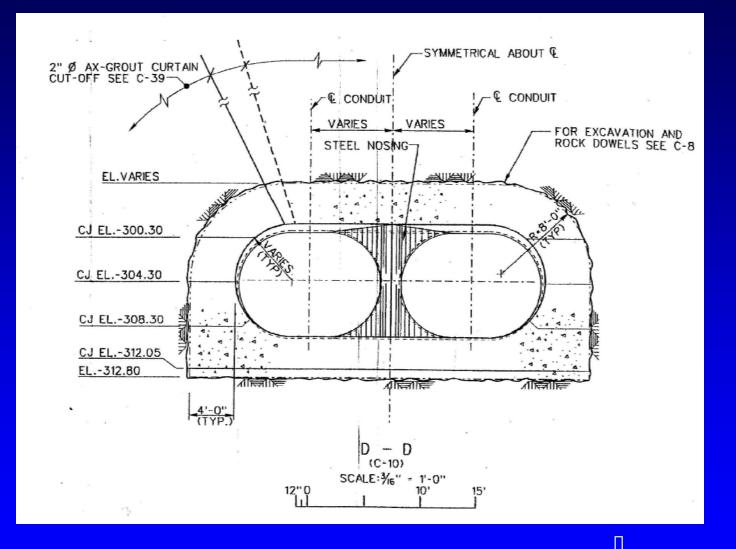
Plan - Concrete Bifurcation





Plan of Concrete Bifurcation

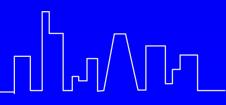






Hydraulic Design Consideration

Concrete Bifurcation is subjected to moderate turbulence - 10,000 psi concrete





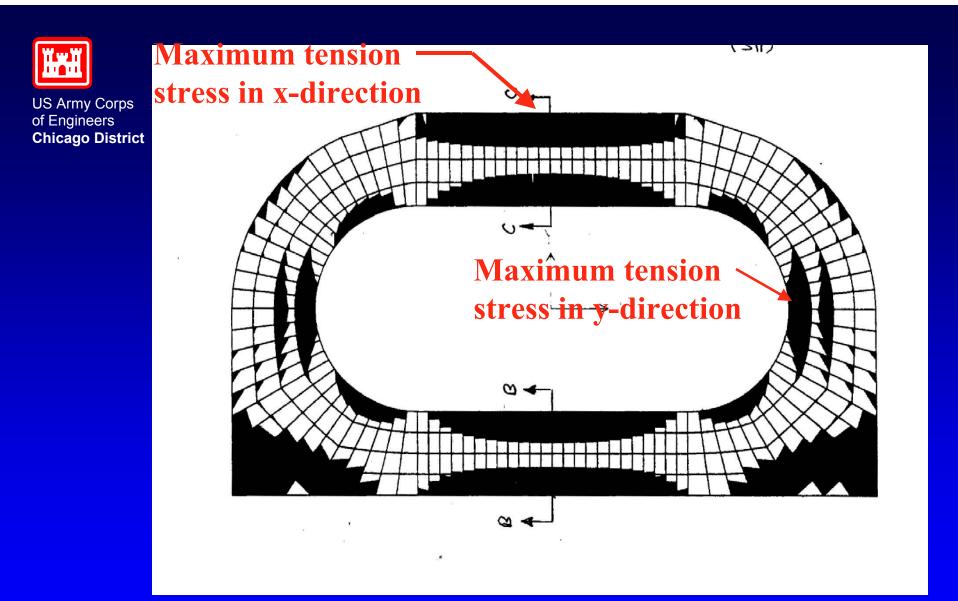
External Pressure Design

- Designed for external pressure of 136 psi
- External Pressures are resisted by the use of rock anchors on all sides
 necessary due to non-circular shape
- Concrete sections are designed per ACI 318.



Internal Pressure Design

- Designed for internal pressure of 160 psi
- SAP 2000 was used for the Analyses to include the effects of the surrounding rock mass. Similar to tunnel design.
- Concrete designed for watertightness and allowable crack width of .008 inches



Maximum Stresses – (Internal Pressure)









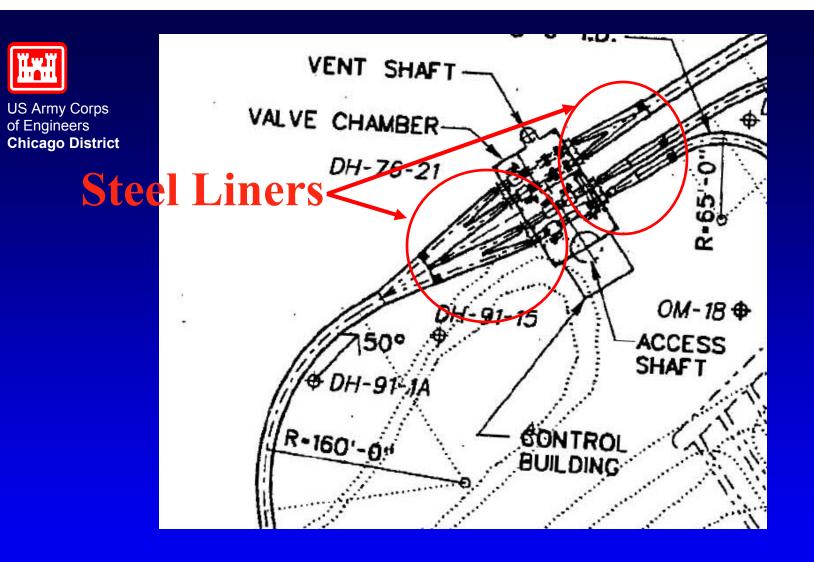






Overview of Steel Liner Design





Steel Liners Located at Distribution Chamber



Purpose of Steel Liners

 Provide erosion protection in areas around Distribution Chamber

- Velocities > 100 fps

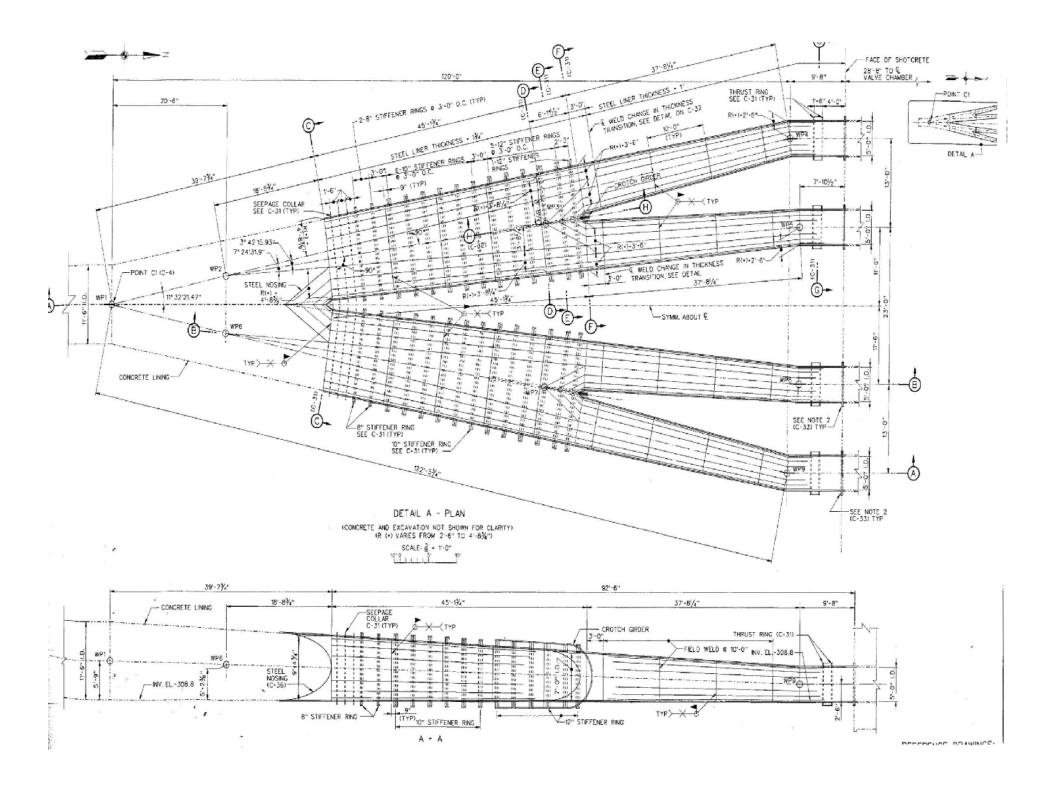
• Form the bifurcation geometry



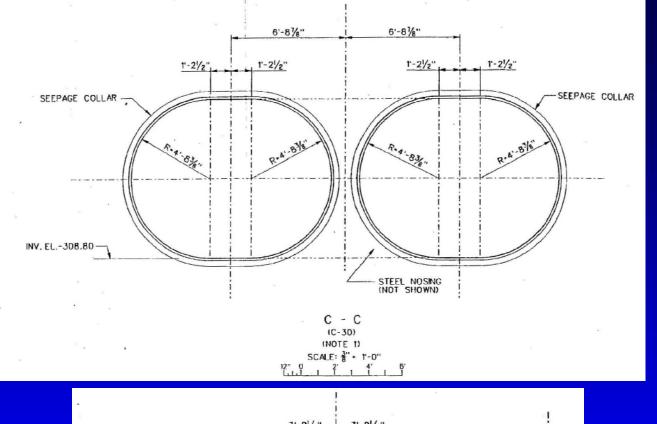
Design of Steel Liners

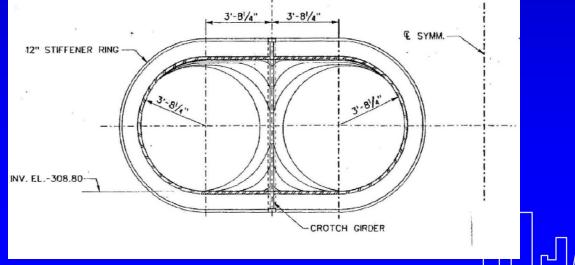
- Designed for internal and external pressures
- Circular Section designed per EM 2901 Section 9-5d.
- ASME Pressure Vessel Code, Section VIII used for design of noncircular sections
- Stiffeners are provided on obround liner sections to resist buckling
- In areas of geometric discontinuities, 3-D STAAD Model used to design the cross sections.

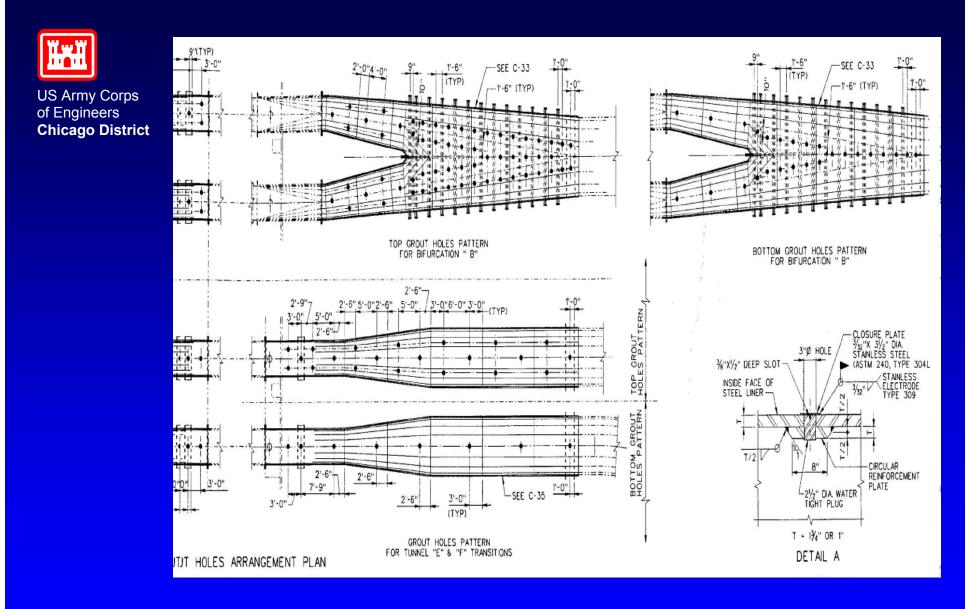




















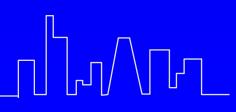
















Steel Nosing being lowered into 26' dia. Access shaft









View From Inside Steel Liner





Steel Liner Being Welded – Oblong Section





Positioning Steel Nosing





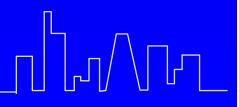




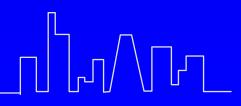




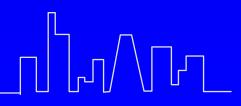
Thank You













Machine-bored Tunnel (the new way)





Intersection of Machine-bored Tunnels





TUNNEL BORING MACHINE







27-ft Diameter Machine-bored Tunnel – Before Lining







Placing Concrete for Tunnel Lining



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LINED TUNNEL

